

DEPARTMENT OF TRANSPORTATION

DIVISION OF ENGINEERING SERVICES

OFFICE ENGINEER

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May 2, 2013

02-Plu-70-37.5/46.2

02-4E9804

Project ID 0212000115

ACSTP-P070(127)E

Addendum No. 1

Dear Contractor:

This addendum is being issued to the contract for CONSTRUCTION ON STATE HIGHWAY IN PLUMAS COUNTY IN AND NEAR QUINCY FROM 0.1 MILE EAST OF OLD COUNTY ROAD TO 0.1 MILE EAST OF ELM STREET.

Submit bids for this work with the understanding and full consideration of this addendum. The revisions declared in this addendum are an essential part of the contract.

Bids for this work will be opened on Wednesday, May 15, 2013.

This addendum is being issued to revise *Notice to Bidders and Special Provisions*.

In the Special Provisions, Section 39, "Hot Mix Asphalt," is replaced as attached.

To *Bid* book holders:

Inquiries or questions in regard to this addendum must be communicated as a bidder inquiry and must be made as noted in the *Notice to Bidders* section of the *Notice to Bidders and Special Provisions*.

Indicate receipt of this addendum by filling in the number of this addendum in the space provided on the signature page of the *Bid* book.

Submit bids in the *Bid* book you now possess. Holders who have already mailed their book will be contacted to arrange for the return of their book.

Inform subcontractors and suppliers as necessary.

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May 2, 2013

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This addendum and attachments are available for the Contractors' download on the Web site:

http://www.dot.ca.gov/hq/esc/oe/project_ads_addenda/02/02-4E9804

If you are not a *Bid* book holder, but request a book to bid on this project, you must comply with the requirements of this letter before submitting your bid.

Sincerely,



JOHN BULINSKI
District Director

Attachments

39 HOT MIX ASPHALT

Add to section 39-1.01:

Produce and place HMA Type A under the Standard construction process.

Add to section 39-1.01A:

For HMA Type A, B, and HMA with warm mix asphalt technology do not pave on the traveled way between November 1 and May 1 if:

1. The quantity of HMA is greater than 1000 tons or
2. The project elevation is greater than 1500 feet

For HMA-O, RHMA-G, RHMA-O, or RHMA-O-HB do not pave on the traveled way between September 15 and May 1.

Replace the 1st paragraph of section 39-1.02B with:

Tack coat must comply with the specifications for asphaltic emulsion or asphalts. Use CRS2, CQS1, asphalt binder, or PMCRS2 asphaltic emulsion.

Add to section 39-1.02C:

Asphalt binder used in HMA Type A must be PG 64-28 M.

Add to section 39-1.02E:

Aggregate used in HMA Type A must comply with the 1/2-inch HMA Types A and B gradation.

If aggregate source is from Modoc, Siskiyou, or Shasta County, submit aggregate samples to the Engineer at least 30 days before the aggregate's intended use.

Treat HMA aggregate with lime using the slurry method.

Add to the 4th table of section 39-1.02E:

Aggregate Quality

Quality characteristic	Test method	HMA type			
		A	B	RHMA-G	OGFC
Sodium sulfate soundness (% max loss) ^c	California Test 214	25	25	25	25
Coarse durability index (min) ^d	California Test 229	65	65	65	65
Fine durability index (min)	California Test 229	50	50	50	50
Plasticity Index	California Test 204	<10	<10	<10	--

^c Requirement applies only if aggregate source is from Modoc, Siskiyou, or Shasta County.

^d Requirement applies only if aggregate source is from Lassen, Modoc, Siskiyou or Shasta County.

Replace the 2nd row of the 4th table of section 39-1.02E with:

Aggregate Quality

Quality characteristic	Test method	HMA type			
		A	B	RHMA-G	OGFC
Los Angeles Rattler (% max.) Loss at 100 rev.	California Test 211	12	--	12	12
Loss at 500 rev.		25	25	25	25

Replace the 3rd paragraph of section 39-1.03A with:

Laboratories testing aggregate qualities, RAP, and preparing the mix design and JMF must be qualified under the Department's Independent Assurance Program. Take samples under California Test 125.

When doing your mix design take three 80 lb RAP samples from stockpiles under California Test 125. Split each sample into 2 parts:

1. Each part must weigh at least 40 lb.
2. Submit 1 part to the Engineer with the JMF.
3. Use 1 part for your testing.

Add to the 1st table of the RSS for section 39-1.03B:

HMA Mix Design Requirements

Quality characteristic	Test method	HMA type		
		A	B	RHMA-G
Optimum Bitumen Content (OBC)	California Test 367	--	--	7.0% min
Moisture susceptibility (minimum dry strength, psi)	California Test 371	100	100	100
Moisture susceptibility (tensile strength ratio, %) ^b	California Test 371	≥80	≥80	≥80

^b After lime treatment.

^c Determine the following using AASHTO T84: bulk specific gravity (SSD) of fine aggregate, bulk specific gravity (oven dry) of fine aggregate, G_r , and absorption of fine aggregate.

Replace the 1st and 2nd rows of the 1st table of the RSS for section 39-1.03B with:

HMA Mix Design Requirements

Quality characteristic	Test method	HMA type		
		A	B	RHMA-G
Air void content (%)	California Test 367	4.0	4.0	3.5
Voids in mineral aggregate (% min.)	California Test 367 ^c			
No. 4 grading		17.0	17.0	--
3/8" grading		15.0	15.0	--
1/2" grading		14.0	14.0	18.0–23.0
3/4" grading		13.0	13.0	18.0–23.0

Replace the 2nd table in the RSS for section 39-1.03B with:

**Additional HMA Mix Design Requirements
for RAP Substitution Rate Greater Than 15 Percent**

Quality characteristic	Test method	HMA type		
		A	B	RHMA-G
Hamburg wheel track (minimum number of passes at 0.5 inch average rut depth)	AASHTO T 324 (Modified)			
PG-58		10,000	10,000	--
PG-64		15,000	15,000	
PG-70		20,000	20,000	
PG-76 or higher		25,000	25,000	
Hamburg wheel track (inflection point minimum number of passes)	AASHTO T 324 (Modified)			
PG-58		10,000	10,000	--
PG-64		10,000	10,000	
PG-70		12,500	12,500	
PG-76 or higher		15,000	15,000	

Add to section 39-1.03B:

If the project is greater than 1500 feet elevation, perform a mix design that produces the quality characteristic shown in the table when mixed with the asphalt used on the project in the amount determined to be optimum by California Test 367:

Quality Characteristic	Test	Requirement
Surface abrasion	California Test 360	Loss not to exceed 0.4 g/cm ²

Replace the 4th and 5th paragraphs of section 39-1.03C with:

For HMA Type A, B or RHMA-G submit test results with the JMF submittal for:

1. California Test 204 plasticity index
2. California Test 371 for treated and untreated HMA for:
 - 2.1. Tensile strength ratio
 - 2.2. Minimum dry strength
3. AASHTO T 324 (Modified), for RAP substitution greater than 15 percent

Replace the 6th paragraph of section 39-1.03C with:

For HMA Type A, B or RHMA-G submit the California Test 371 tensile strength ratio, California Test 371 minimum dry strength, and AASHTO T 324 (Modified) test results to:

1. The Engineer
2. Moisture_Tests@dot.ca.gov

Delete the last two paragraphs of the RSS for Section 39-1.03C.

Delete the last paragraph of section 39-1.03E.

Delete "If required," from the 5th item in the 2nd paragraph of the RSS for section 39-1.03G.

Replace the 3rd paragraph of the RSS for section 39-1.03G with:

With an accepted modified JMF submittal, the Engineer verifies each modified JMF within 10 business days of receiving all verification samples.

Replace the 4th paragraph of the RSS for section 39-1.03G with:

The Engineer verifies the modified JMF after the modified JMF HMA is placed on the project and verification samples are taken within the first 750 tons following sampling requirements in section 39-1.03E, "Job Mix Formula Verification." The Engineer tests verification samples for compliance with:

1. Stability as shown in the table titled "HMA Mix Design Requirements"
2. Air void content at
 - 2.1. Design value ± 2.0 percent for HMA Type A and Type B
 - 2.2. Design value ± 1.5 percent for RHMA-G
3. Voids in mineral aggregate as shown in the table titled "HMA Mix Design Requirements"
4. Voids filled with asphalt, report only
5. Dust proportion, report only
6. Optimum Bitumen Content
7. Tensile Strength Ratio
8. Minimum Dry Strength
9. Hamburg Wheel Track for RAP substitution greater than 15 percent, as shown in the table titled "Additional HMA Mix Design Requirements for RAP Substitution Rate Greater Than 15 Percent"

Replace the last paragraph of the RSS for section 39-1.03G with:

The Engineer deducts \$4,000 from payments for each modified JMF verification.

Add to section 39-1.08A:

On the first production day and once during production of the first 5,000 tons, submit:

1. Samples split from your HMA production sample for California Test 371 to:
 - 1.1 The Engineer
 - 1.2 The Transportation Laboratory, Attention: Moisture Test.
2. The California Test 371 results to:
 - 2.1 The Engineer
 - 2.2 Moisture_Tests@dot.ca.gov

After the 1st production day and production of the first 5,000 tons, submit the California Test 371 results for each 5,000 tons to:

1. The Engineer
2. Moisture_Tests@dot.ca.gov

Add to section 39-1.11A of the RSS for section 39-1.11:

Place RHMA-G only when the atmospheric temperature is 70 degrees F or greater.

Use a material transfer vehicle (MTV) if:

1. The project quantity of hot mix asphalt to be paved is greater than 1000 tons, and
2. Any of the following exists:
 - 2.1. Paving is allowed and the atmospheric temperature is below 70 degrees F.
 - 2.2. Time from discharge to truck at the HMA plant until transfer to the paver's hopper is 90 minutes or greater.

The MTV must:

1. Either receive HMA directly from the truck or use a pickup head to load it from a windrow than can be deposited on the roadway surface for a maximum of 100 feet in length.
2. Remix the HMA, with augers, before loading the paver.
3. Transfer HMA directly into the paver's receiving hopper or feed system.
4. Have sufficient capacity to prevent stopping the paver.

The MTV requirements will not apply to replace asphalt concrete surfacing under section 39-1.21.

Remove pavement markers prior to applying tack coat or placing HMA.

If there is no bid item for remove pavement markers, removing pavement markers is included in HMA (Type A).

Replace the 2nd, 3rd, and 4th paragraphs of section 39-1.11B(1) of the RSS for section 39-1.11 with:

Place HMA on adjacent traveled way lanes so that at the end of each work shift the distance between the ends of HMA layers on adjacent lanes is from 5 to 10 feet. Place additional HMA along the transverse edge at each lane's end and along the exposed longitudinal edges between adjacent lanes. Hand rake and compact the additional HMA to form temporary conforms. You may place Kraft paper or another authorized bond breaker under the conform tapers to facilitate the taper removal when paving operations resume.

Delete section 39-1.11B(2) of the RSS for section 39-1.11.

Replace the headings and paragraphs in section 39-1.12 with:

Replace section 39-1.12E of the RSS for section 39-1.12 with:

39-1.12E Prepaving Must-Grinds

Section 39-1.12E applies to existing asphalt concrete areas receiving an HMA overlay of less than 0.25 foot.

Prepaving profilograph includes taking profiles of the existing pavement, determining must-grinds, and submitting profilograms.

Prepaving grinding day includes correcting must-grinds and taking profiles of the corrected areas and submitting profilograms.

Before starting paving operations, determine must-grinds on the existing pavement under California Test 526. Take 2 profiles within each traffic lane, 3 feet from and parallel with the edge of each lane. Profile the pavement in the Engineer's presence.

Submit profilograms and locations of must-grinds.

Notify the Engineer of those must-grinds that cannot be corrected by prepaving grinding. The Engineer responds to your notification within 5 business days.

For those must-grinds that cannot be corrected by grinding, the Engineer may order you to either (1) not correct the must-grinds or (2) correct must-grinds by a different method and take profiles of the corrected areas with a profilograph.

Corrective work not performed by prepaving grinding, including taking profiles of the corrected areas and associated traffic control, is change order work.

Correct prepaving must-grinds that you predict will cause the top layer of HMA to be noncompliant with the smoothness specifications. After correcting prepaving must-grinds, take profiles of the corrected area and submit profilograms.

Dispose of grinding residue.

Pave within 7 days of correcting areas.

The HMA pavement top layer must comply with section 39-1.12. The 2nd paragraph of section 39-1.12A and the 3rd paragraph of section 39-1.12C do not apply regardless of the type of HMA used.

If ordered not to correct prepaving must-grinds, the smoothness specifications do not apply to the top layer of HMA placed in those areas.

Smoothness correction of the top layer of HMA must leave at least 75 percent of the specified HMA thickness. If ordered, core the pavement at the locations determined by the Engineer. Coring, including traffic control, is change order work. Remove and replace deficient pavement areas where the overlay thickness is less than 75 percent of the thickness specified as determined by the Engineer.

39-1.12F Smoothness Correction

If the final surface of the pavement does not comply with section 39-1.12D, grind the pavement to within specified tolerances, remove and replace it, or place an overlay of HMA. Do not start corrective work until your method is authorized.

Smoothness correction of the final pavement surface must leave at least 75 percent of the specified HMA thickness. If ordered, core the pavement at the locations determined by the Engineer. Coring, including traffic control, is change order work. Remove and replace deficient pavement areas where the overlay thickness is less than 75 percent of the thickness specified as determined by the Engineer.

If you choose to correct OGFC, the Engineer determines if the corrective method causes raveling. OGFC that is raveling must be removed and replaced.

Corrected HMA pavement areas must be uniform rectangles with edges:

1. Parallel to the nearest HMA pavement edge or lane line
2. Perpendicular to the pavement centerline

On ground areas not to be overlaid with OGFC, apply fog seal coat under section 37-2.

Where corrections are made within areas requiring testing with IP, reprofile the entire lane length with the IP device.

Where corrections are made within areas requiring testing with a 12-foot straightedge, retest the corrected area with the straightedge.

39-1.12H Prepaving Grinding

Section 39-1.12H applies to existing asphalt concrete areas receiving an HMA overlay of less than or equal to 0.20 foot.

Correct areas of localized roughness greater than 140 in/mi.

Prepaving grinding day includes correcting areas of localized roughness, taking profiles of the corrected areas, and submitting profile data as specified in section 39-1.12B.

Notify the Engineer of those areas of localized roughness that cannot be corrected by prepaving grinding. The Engineer responds to your notification within 5 business days.

For those areas of localized roughness that cannot be corrected by grinding, the Engineer may order you to either (1) not correct the areas of localized roughness or (2) correct areas of localized roughness by a different method and take profiles of the corrected areas with an IP.

Corrective work not performed by prepaving grinding, including taking profiles of the corrected areas and associated traffic control, is change order work.

Correct prepaving areas of localized roughness that you predict will cause the final surface of HMA pavement to be noncompliant with the smoothness specifications. After correcting prepaving areas of localized roughness, take profiles of the corrected area and submit profile data as specified in section 39-1.12B.

Dispose of grinding residue.

Pave within 7 days of correcting areas.

The final pavement surface must comply with section 39-1.12D.

If ordered not to correct areas of localized roughness, the smoothness specifications do not apply to the final pavement surface placed in those areas.

Replace section 39-1.19 with:

39-1.19 HOT MIX ASPHALT AGGREGATE LIME TREATMENT—SLURRY METHOD

39-1.19A General

39-1.19A(1) Summary

Treat HMA aggregate with lime using the slurry method and place it in stockpiles to marinate.

39-1.19A(2) Submittals

Determine the exact lime proportions for treated aggregate stockpiles and resulting combined aggregate. Submit them as part of the proposed JMF.

Submit the averaged aggregate quality test results to the Engineer within 24 hours of sampling.

Submit a treatment data log from the slurry proportioning device in the following order:

1. Treatment date
2. Time of day the data is captured
3. Aggregate size being treated
4. Wet aggregate flow rate collected directly from the aggregate weigh belt
5. Moisture content of the aggregate just before treatment, expressed as a percent of the dry aggregate weight
6. Dry aggregate flow rate calculated from the wet aggregate flow rate
7. Lime slurry flow rate measured by the slurry meter
8. Dry lime flow rate calculated from the slurry meter output
9. Authorized lime ratio for each aggregate size being treated
10. Actual lime ratio calculated from the aggregate weigh belt and the slurry meter output, expressed as a percent of the dry aggregate weight
11. Calculated difference between the authorized lime ratio and the actual lime ratio
12. Dry lime and water proportions at the slurry treatment time

Every day during lime treatment, submit the treatment data log on electronic media in tab delimited format on a removable CD-ROM storage disk. Each continuous treatment data set must be a separate record using a line feed carriage return to present the specified data on 1 line. The reported data must include data titles at least once per report.

39-1.19A(3) Quality Control and Assurance

The QC plan must include aggregate quality control sampling and testing during aggregate lime treatment. Sample and test in compliance with frequencies in the following table:

Quality characteristic	Test method	Minimum sampling and testing frequency
Sand equivalent	California Test 217	Once per 1,000 tons of aggregate treated with lime
Course durability index (D _c) (min) ^e	California Test 229	1 per 3,000 tons of aggregate treated with lime
Fine durability index (D _f) (min)	California Test 229	
Percent of crushed particles	California Test 205	As necessary and as designated in the QC plan
Los Angeles Rattler	California Test 211	
Fine aggregate angularity	California Test 234	
Flat and elongated particles	California Test 235	

Note: During lime treatment, sample coarse and fine aggregate from individual stockpiles. Combine aggregate in the JMF proportions. Run tests for aggregate quality in triplicate and report test results as the average of 3 tests.

^eRequirement applies only if aggregate source is from Lassen, Modoc, Siskiyou or Shasta County.

For any of the following, the Engineer orders proportioning operations stopped if you:

1. Do not submit the treatment data log
2. Do not submit the aggregate quality control data
3. Submit incomplete, untimely, or incorrectly formatted data
4. Do not take corrective actions
5. Take late or unsuccessful corrective actions
6. Do not stop treatment when proportioning tolerances are exceeded
7. Use malfunctioning or failed proportioning devices

If you stop treatment, notify the Engineer of any corrective actions taken and conduct a successful 20-minute test run before resuming treatment.

For the aggregate to be treated, determine the moisture content at least once during each 2 hours of treatment. Calculate moisture content under California Test 226 or 370 and report it as a percent of dry aggregate weight. Use the moisture content calculations as a set point for the proportioning process controller.

39-1.19B Materials

High-calcium hydrated lime and water must comply with section 24-2.02.

Before virgin aggregate is treated, it must comply with the aggregate quality specifications. Do not test treated aggregate for quality control except for gradation. The Engineer does not test treated aggregate for acceptance except for gradation.

The Engineer determines the combined aggregate gradation during HMA production after you have treated the aggregate. If RAP is used, the Engineer determines combined aggregate gradations containing RAP under Laboratory Procedure LP-9.

Treated aggregate must not have lime balls or clods.

39-1.19C Construction

39-1.19C(1) General

Notify the Engineer at least 24 hours before the start of aggregate treatment.

Treat aggregate separate from HMA production.

Do not treat RAP.

Add lime to the aggregate as slurry consisting of mixed dry lime and water at a ratio of 1 part lime to from 2 to 3 parts water by weight. The slurry must completely coat the aggregate.

Lime treat and marinate coarse and fine aggregate stockpiles separately.

Immediately before mixing lime slurry with the aggregate, water must not visibly separate from the aggregate.

Treat the aggregate and stockpile for marination only once.

The lime ratio is the pounds of dry hydrated lime per 100 lb of dry virgin aggregate expressed as a percentage. Water content of slurry or untreated aggregate must not affect the lime ratio.

The following aggregate gradations must have the lime ratio ranges shown in the following table:

Aggregate gradation	Lime ratio percent
Coarse virgin stockpiles ^a	0.4–1.0
Fine virgin stockpiles ^a	1.5–2.0
Combined virgin aggregate	1.0–1.5

^a Stockpiles containing predominately coarse aggregate are coarse aggregate stockpiles. Stockpiles containing predominately fine aggregate are fine aggregate stockpiles.

For OGFC, you may reduce the combined virgin aggregate lime ratio to 0.5–1.0 percent.

The lime ratio for fine and coarse virgin aggregate stockpiles must be within ± 0.2 percent of the lime ratio in the accepted JMF. The lime ratio must be within ± 0.2 percent of the authorized lime ratio when you combine the individual aggregate sizes in the JMF proportions. The lime ratio must be determined before the addition of RAP.

If 3 consecutive sets of recorded treatment data indicate deviation more than 0.2 percent above or below the lime ratio in the accepted JMF, stop treatment.

If a set of recorded treatment data indicates a deviation of more than 0.4 percent above or below the lime ratio in the accepted JMF, stop treatment and do not use the material represented by that set of data in HMA.

If 20 percent or more of the total daily treatment indicates deviation of more than 0.2 percent above or below the lime ratio in the accepted JMF, stop treatment and do not use the day's total treatment in HMA.

If you stop treatment for noncompliance, you must implement corrective action and successfully treat aggregate for a 20-minute period. Notify the Engineer before beginning the 20-minute treatment period.

39-1.19C(2) Lime Slurry Proportioning

Proportion lime and water with a continuous or batch operation.

The device controlling slurry proportioning must produce a treatment data log. The log consists of a series of data sets captured at 10-minute intervals throughout daily treatment. The data must be a treatment activity register and not a summation. The material represented by the data set is the quantity produced 5 minutes before and 5 minutes after the capture time. For the Contract's duration, collected data must be stored by the controller.

39-1.19C(3) Proportioning and Mixing Lime Slurry Treated Aggregate

Treat HMA aggregate by proportioning lime slurry and aggregate by weight in a continuous operation.

Marinate treated aggregate in stockpiles from 24 hours to 60 days before using in HMA. Do not use aggregate marinated longer than 60 days.

39-1.19D Payment

Payment for treating aggregates with lime slurry is included in payment for the HMA involved.

Replace section 39-1.21 with:

39-1.21 REPLACE ASPHALT CONCRETE SURFACING

39-1.21A General

Remove existing asphalt concrete surfacing and underlying base and replace with HMA. The Engineer determines the exact limits of replaced asphalt concrete surfacing.

39-1.21B Materials

HMA for replace asphalt concrete surfacing must be Type A.

Asphalt binder for the HMA must be PG 64-28 M.

The aggregate for the HMA must comply with the 3/4-inch grading.

39-1.21C Construction

Place replacement HMA under section 39-3.

Replace asphalt concrete in a lane before the lane is specified to be opened to traffic under section 12-4.

Before removing asphalt concrete, outline the replacement area and cut neat lines with a saw or grind to full depth of the existing asphalt concrete. Do not damage asphalt concrete and base remaining in place.

Dispose of removed material.

If the base is excavated beyond the specified plane, replace it with HMA. The Department does not pay for this HMA.

39-1.21D Payment

Replace asphalt concrete surfacing is measured based on the specified dimensions and any adjustments ordered.

You may request authorization to leave rejected replacement HMA in place. If authorized, you must accept a reduction in the payment for the rejected replacement HMA.

Add to the first table of the RSS for section 39-2.02B:

Minimum Quality Control—Standard Construction Process

Quality characteristic	Test method	Minimum sampling and testing frequency	HMA type			
			A	B	RHMA-G	OGFC
Coarse durability index (min) ^{k, m}	California Test 229	1 per 3,000 tons during production, but not less than 1 per paving day	65	65	65	65
Fine durability index (min) ^k	California Test 229		50	50	50	50

^k Obtain sample from stockpile before lime treatment.

^l Determine the following using AASHTO T84: bulk specific gravity (SSD) of fine aggregate, bulk specific gravity (oven dry) of fine aggregate, G_r , and absorption of fine aggregate.

^m Requirement applies only if aggregate source is from Lassen, Modoc, Siskiyou or Shasta County.

Replace the 7th, 10th, 14th, 18th and 19th rows of the first table of the RSS for section 39-2.02B with:

Minimum Quality Control—Standard Construction Process

Quality characteristic	Test method	Minimum sampling and testing frequency	HMA type			
			A	B	RHMA-G	OGFC
Air void content (%) ^{c, f}	California Test 367	1 per 4,000 tons or 2 per 5 business days, whichever is greater	4 ± 2	4 ± 2	3.5 ± 1.5	--
Los Angeles Rattler (% max) ^k Loss at 100 rev. Loss at 500 rev.	California Test 211	1 per 3,000 tons during production, but not less than 1 per paving day	12 25	-- 25	12 25	12 25
Voids in mineral aggregate(% min) ⁱ No. 4 grading 3/8" grading 1/2" grading 3/4" grading	California Test 367 ^l		17.0 15.0 14.0 13.0	17.0 15.0 14.0 13.0	-- -- 18.0–23.0 18.0–23.0	-- --
Moisture susceptibility (minimum dry strength, psi)	California Test 371	1 per 10,000 tons or 1 per project whichever is greater	100	100	100	--
Moisture susceptibility (tensile strength ratio, %)	California Test 371	1 per 10,000 tons or 1 per project whichever is greater	80	80	80	--

CONTRACT NO. 02-4E9804

REPLACED PER ADDENDUM NO. 1 DATED MAY 2, 2013

Add to the first table of the RSS for section 39-2.03A:

HMA Acceptance—Standard Construction Process

Quality characteristic	Test method	HMA type			
		A	B	RHMA-G	OGFC
Coarse durability index (min) ^{k, m}	California Test 229	65	65	65	65
Fine durability index (min) ^k	California Test 229	50	50	50	50

^k Obtain sample from stockpile before lime treatment.

^l Determine the following using AASHTO T84: bulk specific gravity (SSD) of fine aggregate, bulk specific gravity (oven dry) of fine aggregate, G_r , and absorption of fine aggregate.

^m Requirement applies only if aggregate source is from Lassen, Modoc, Siskiyou or Shasta County.

Replace the 7th, 9th, 13th, 17th and 18th rows of the first table of the RSS for section 39-2.03A with:

HMA Acceptance—Standard Construction Process

Quality characteristic	Test method	HMA type			
		A	B	RHMA-G	OGFC
Air void content (%) ^{d, g}	California Test 367	4 ± 2	4 ± 2	3.5 ± 1.5	--
Los Angeles Rattler (% max) ^k	California Test 211				
Loss at 100 rev.		12	--	12	12
Loss at 500 rev.		25	25	25	25
Voids in mineral aggregate (% min) ⁱ	California Test 367 ^l				
No. 4 grading		17.0	17.0	--	--
3/8" grading		15.0	15.0	--	--
1/2" grading		14.0	14.0	18.0–23.0	
3/4" grading		13.0	13.0	18.0–23.0	
Moisture susceptibility (minimum dry strength, psi)	California Test 371	100	100	100	--
Moisture susceptibility (tensile strength ratio, %)	California Test 371	80	80	80	--

Add to the first table of the RSS for section 39-3.02A:

HMA Acceptance—Method Construction Process

Quality characteristic	Test method	HMA type			
		A	B	RHMA-G	OGFC
Coarse durability index (min) ^{h,j}	California Test 229	65	65	65	65
Fine durability index (min) ^h	California Test 229	50	50	50	50

^h Obtain sample from stockpile before lime treatment.

ⁱ Determine the following using AASHTO T84: bulk specific gravity (SSD) of fine aggregate, bulk specific gravity (oven dry) of fine aggregate, G_r , and absorption of fine aggregate.

^j Requirement applies only if aggregate source is from Lassen, Modoc, Siskiyou or Shasta County.

Replace the 7th, 12th, 16th and 17th rows of the first table of the RSS for section 39-3.02A with:

HMA Acceptance—Method Construction Process

Quality characteristic	Test method	HMA type			
		A	B	RHMA-G	OGFC
Los Angeles Rattler(% , max) ^h	California Test 211				
Loss at 100 rev.		12	--	12	12
Loss at 500 rev.		25	25	25	25
Voids in mineral aggregate (% min) ^f	California Test 367 ⁱ				
No. 4 grading		17.0	17.0	--	--
3/8" grading		15.0	15.0	--	--
1/2" grading		14.0	14.0	18.0-23.0	
3/4" grading		13.0	13.0	18.0-23.0	
Moisture susceptibility (minimum dry strength, psi)	California Test 371	100	100	100	--
Moisture susceptibility (tensile strength ratio, %)	California Test 371	80	80	80	--

Add to the first table of the RSS for section 39-4.02C:

Minimum Quality Control—QC/QA Construction Process

Quality characteristic	Test method	Minimum sampling and testing frequency	HMA Type			Location of sampling	Maximum reporting time allowance
			A	B	RHMA-G		
Coarse durability index(min) ^{k, m}	California Test 229	1 per 3,000 tons during production	65	65	65	Stockpile	48 hours
Fine Durability index (min) ^k	California Test 229	, but not less than 1 per paving day	50	50	50	Stockpile	48 hours

^k Obtain sample from stockpile before lime treatment.

^l Determine the following using AASHTO T84: bulk specific gravity (SSD) of fine aggregate, bulk specific gravity (oven dry) of fine aggregate, G_r, and absorption of fine aggregate.

^m Requirement applies only if aggregate source is from Lassen, Modoc, Siskiyou or Shasta County.

Replace the 8th, 10th, 14th, 18th and 19th rows of the first table of the RSS for section 39-4.02C with:

Minimum Quality Control—QC/QA Construction Process

Quality characteristic	Test method	Minimum sampling and testing frequency	HMA Type			Location of sampling	Maximum reporting time allowance
			A	B	RHMA-G		
Air void content (%) ^{f, g}	California Test 367	1 per 4,000 tons or 2 per 5 business days, whichever is greater	4 ± 2	4 ± 2	3.5 ± 1.5	Loose Mix Behind Paver See California Test 125	48 hours
Los Angeles Rattler (% max) ^k : Loss at 100 rev. Loss at 500 rev.	California Test 211	1 per 3,000 tons during production, but not less than 1 per paving day	12 25	-- 25	12 25	Stockpile	48 hours
Voids in mineral aggregate (% min.) ⁱ No. 4 grading 3/8" grading 1/2" grading 3/4" grading	California Test 367 ^l	As designated in QC plan. At least once per project.	17.0 15.0 14.0 13.0	17.0 15.0 14.0 13.0	-- -- 18.0–23.0 18.0–23.0	California Test 367 ^l	48 hours
Moisture susceptibility (minimum dry strength, psi)	California Test 371	1 per 10,000 tons or 1 per project whichever is greater	100	100	100	--	
Moisture susceptibility (tensile strength ratio, %)	California Test 371	1 per 10,000 tons or 1 per project whichever is greater	80	80	80	--	

Delete the "i" from the footnote in the quality characteristic column of the 16th row of the 1st table of the RSS for section 39-4.02C.

Add to the first table of the RSS for section 39-4.04A:

HMA Acceptance—QC/QA Construction Process

Index (i)	Quality characteristic	Weighting factor (w)	Test method	HMA type		
				A	B	RHMA-G
	Coarse durability index (min) ^{k, m}		California Test 229	65	65	65
	Fine durability index(min) ^k		California Test 229	50	50	50

^k Obtain sample from stockpile before lime treatment.

^l Determine the following using AASHTO T84: bulk specific gravity (SSD) of fine aggregate, bulk specific gravity (oven dry) of fine aggregate, G_s, and absorption of fine aggregate.

^m Requirement applies only if aggregate source is from Lassen, Modoc, Siskiyou or Shasta County.

Replace the 6th, 9th, 12th, 17th and 18th rows of the first table of the RSS for section 39-4.04A with:

HMA Acceptance—QC/QA Construction Process

Index (i)	Quality characteristic	Weighting factor (w)	Test method	HMA type		
				A	B	RHMA-G
	Air void content (%) ^{f, g}		California Test 367	4 ± 2	4 ± 2	3.5 ± 1.5
	Los Angeles Rattler (% max) ^k		California Test 211			
	Loss at 100 rev.			12	--	12
	Loss at 500 rev.			25	25	25
	Voids in mineral aggregate (% min) ⁱ		California Test 367 ^l			
	No. 4 grading			17.0	17.0	--
	3/8" grading			15.0	15.0	--
	1/2" grading			14.0	14.0	18.0–23.0
	3/4" grading			13.0	13.0	18.0–23.0
	Moisture susceptibility (minimum dry strength, psi)		California Test 371	100	100	100
	Moisture susceptibility (tensile strength ratio %)		California Test 371	80	80	80